



# **Systemy SHM opracowane w AGH w oparciu o niskie częstotliwości**

Akademia Górniczo – Hutnicza  
Katedra Robotyki i Mechatroniki  
Al. Mickiewicza 30, 30-059 Kraków



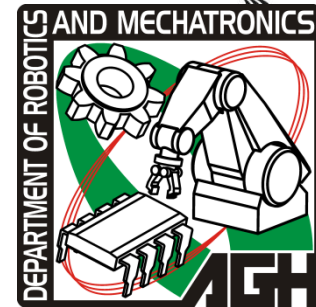
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# Plan prezentacji

- 1. Monitorowanie stanu konstrukcji w oparciu o pomiary wizyjne**
- 2. Implementacja filtru modalnego**



# Monitorowanie stanu konstrukcji w oparciu o pomiarzy wizyjne

Pomiary stanów statycznych  
Pomiary procesów dynamicznych

## *Zespół:*

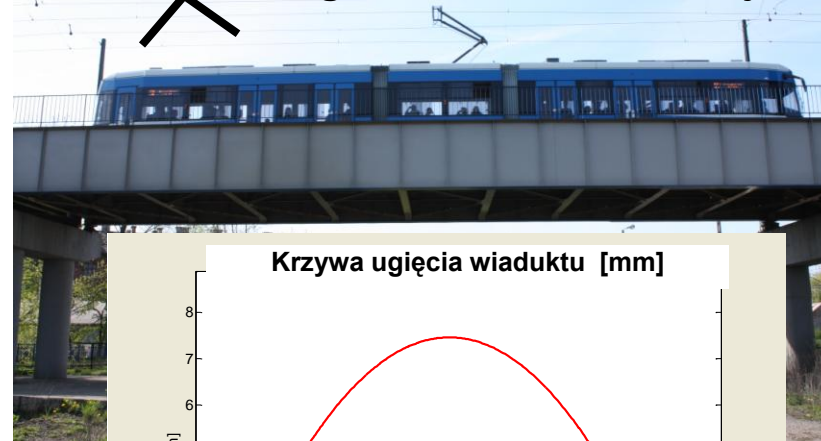
**Prof. dr hab. inż. Uhl Tadeusz,**  
**Dr inż. Kohut Piotr,**  
**Mgr inż. Holak Krzysztof,**  
**Krupiński Krzysztof**  
**Dr inż. Szwedo Mariusz,**



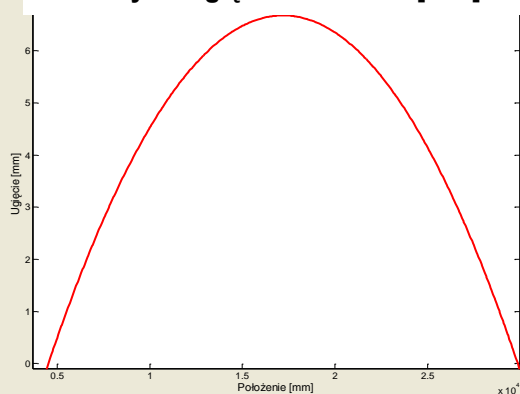
Tani system wykorzystujący ogólnie dostępne aparaty cyfrowe (lustrzanki) do pomiaru odkształceń konstrukcji



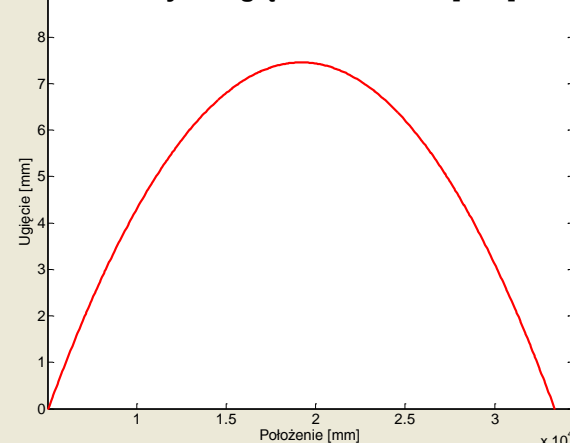
Segment wiaduktu tramwajowego



Krzywa ugięcia wiaduktu [mm]

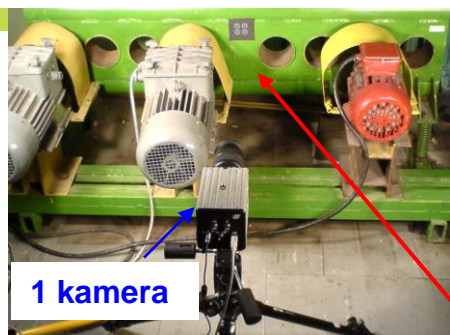


Krzywa ugięcia wiaduktu [mm]



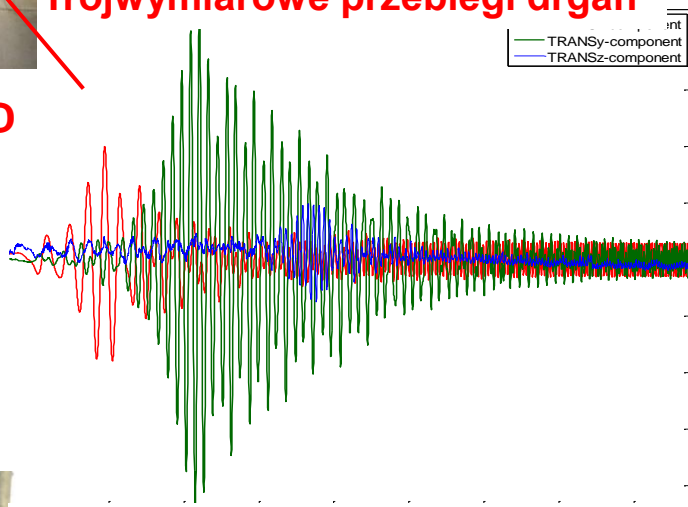
Monitorowane obiekty: mosty, kładki, wiadukty kominy, przęsła, dźwigary, hale, maszty, turbiny wiatrowe, budynki, rurociągi, maszyny i urządzenia

## Maszyna wibracyjna

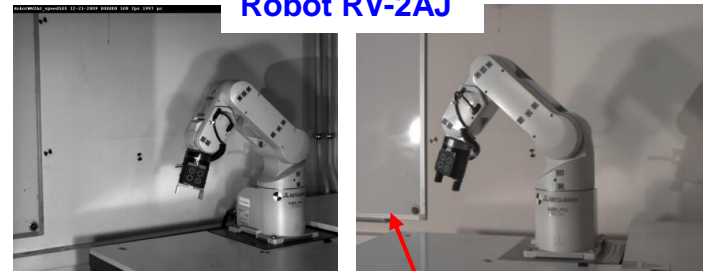


1 kamera

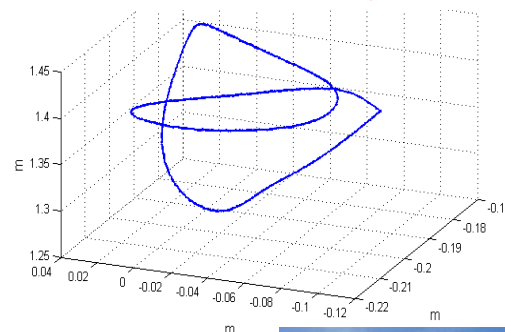
## Trójwymiarowe przebiegi drgań



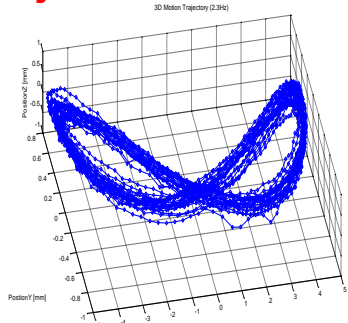
Robot RV-2AJ



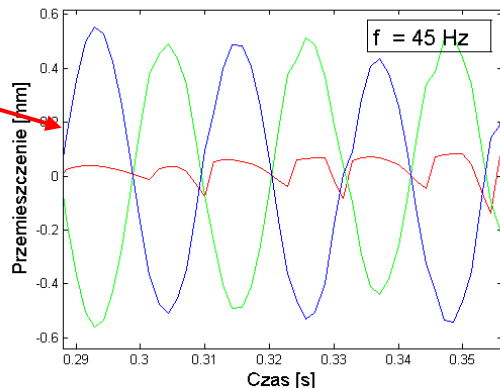
## 3D Trajektoria ruchu chwytaka robota



## Trajektoria ruchu 3D



## Trójwymiarowe przebiegi drgań



przednia Maska samochodu



Monitorowane obiekty: mosty, kładki, wiadukty kominy, przęsła, dźwigary, hale, maszty, turbiny wiatrowe, budynki, rurociągi, maszyny i urządzenia



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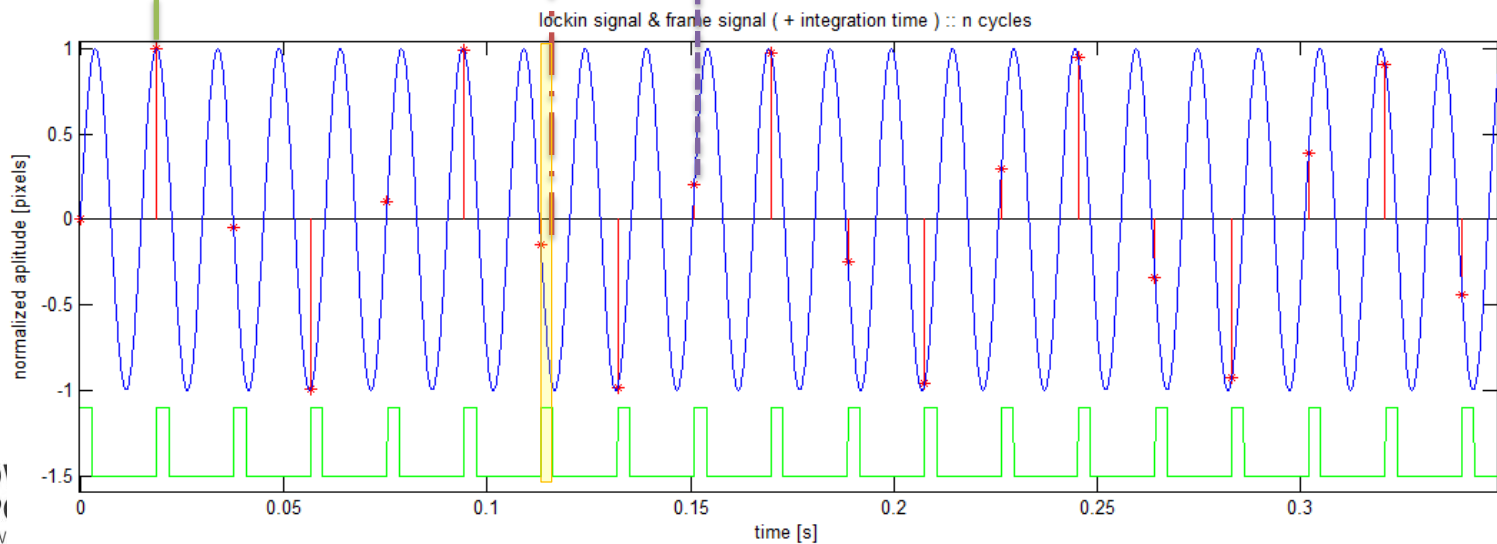
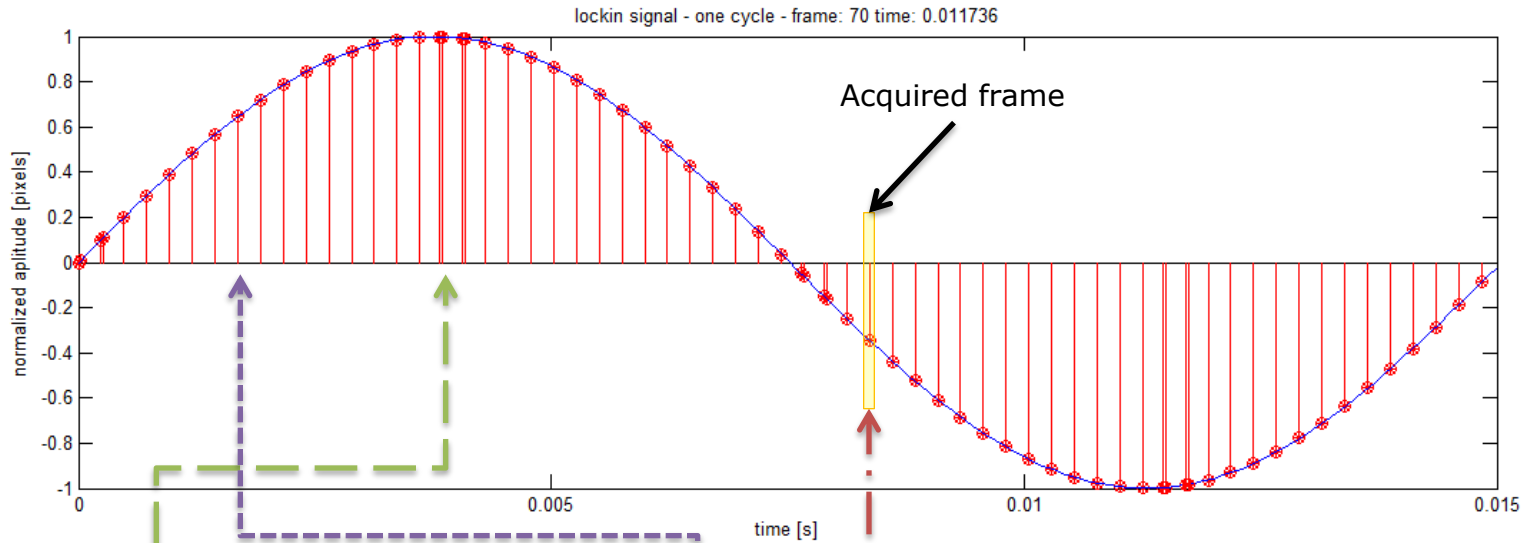
# Lockin signal mixing for modes visualisation based on camera measurement



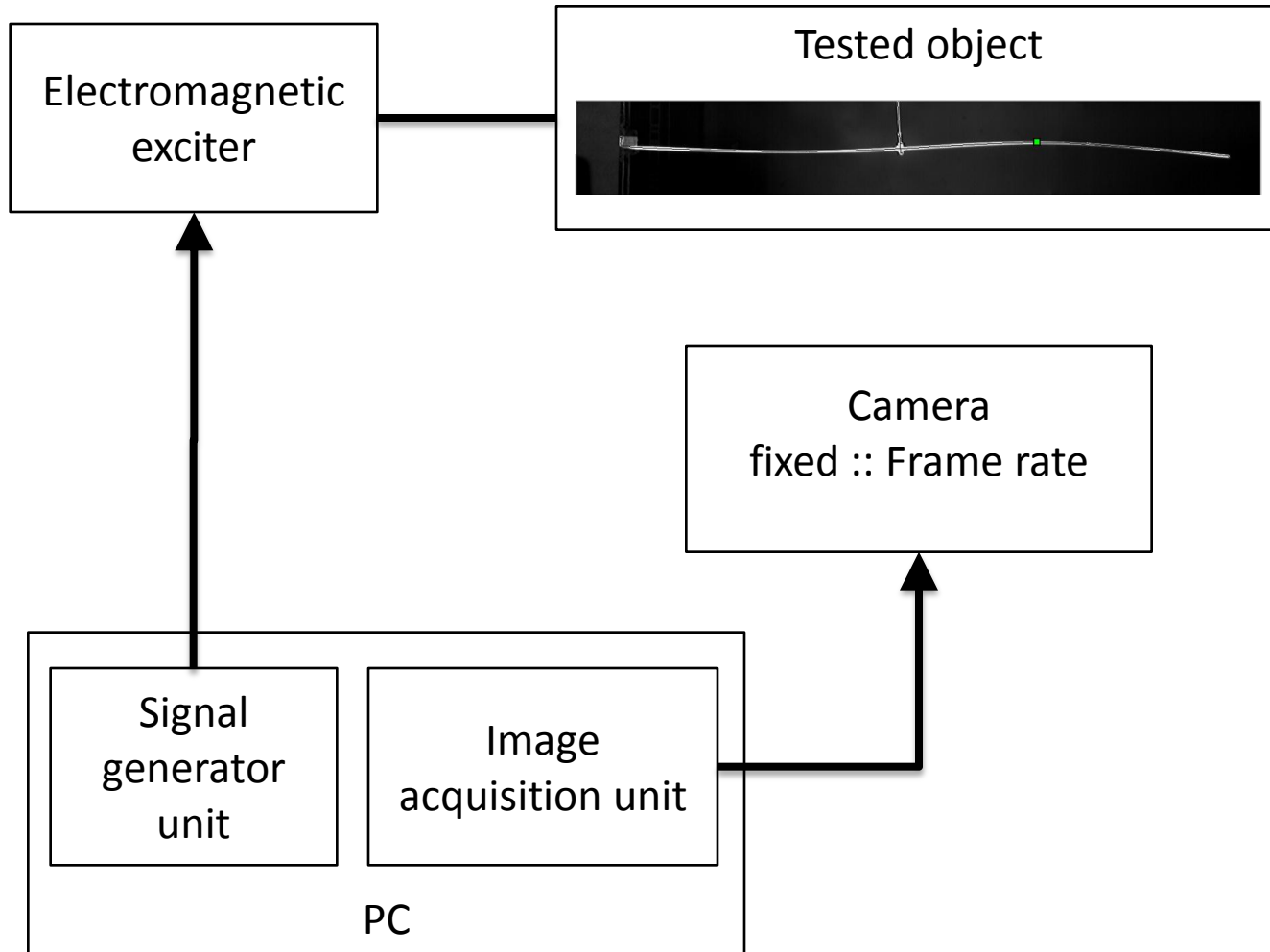
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$$resonance\_frequency > camera\_frame\_rate$$



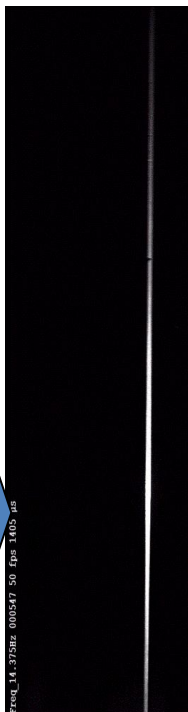
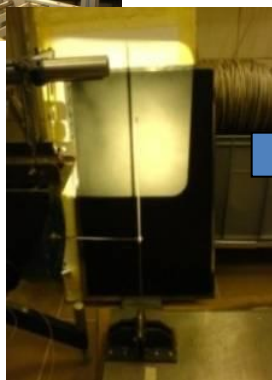
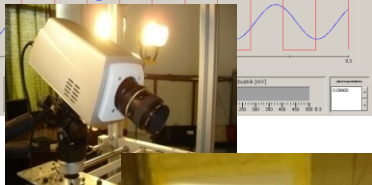




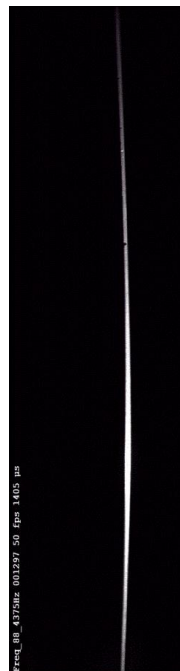
# Wizualizacja postaci drgań analizowanych konstrukcji



Kamera cyfrowa

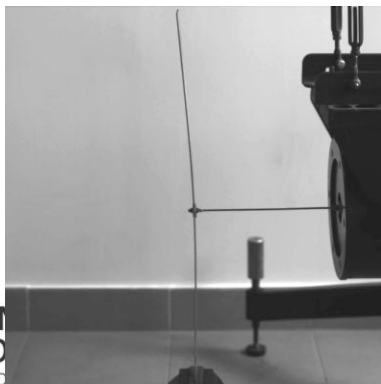


14.375 Hz



88.4375 Hz

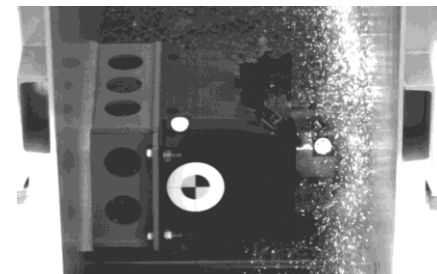
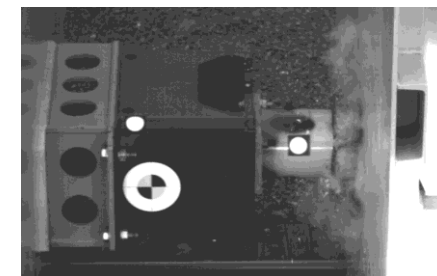
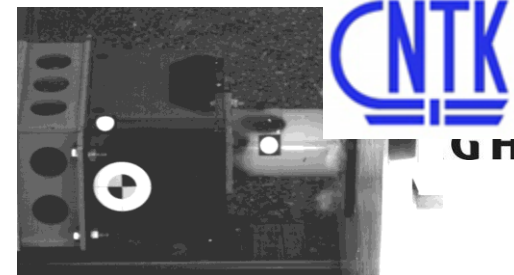
Utwierdzona belka



# Procesy szybkozmienne

Rejestracja pracy absorberów

1000 klatek/sek



Rejestracja pracy wyrobów medycznych - nakłuwaczy

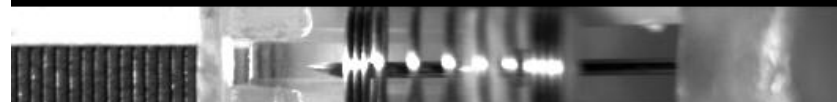
~10 000 klatek/sek



0.375052 s 9660 fps



pzoS 003632 0.375984 s 9660 fps



pzoS 003649 0.377743 s 9660 fps



ROZWOJU REGIONALNEGO



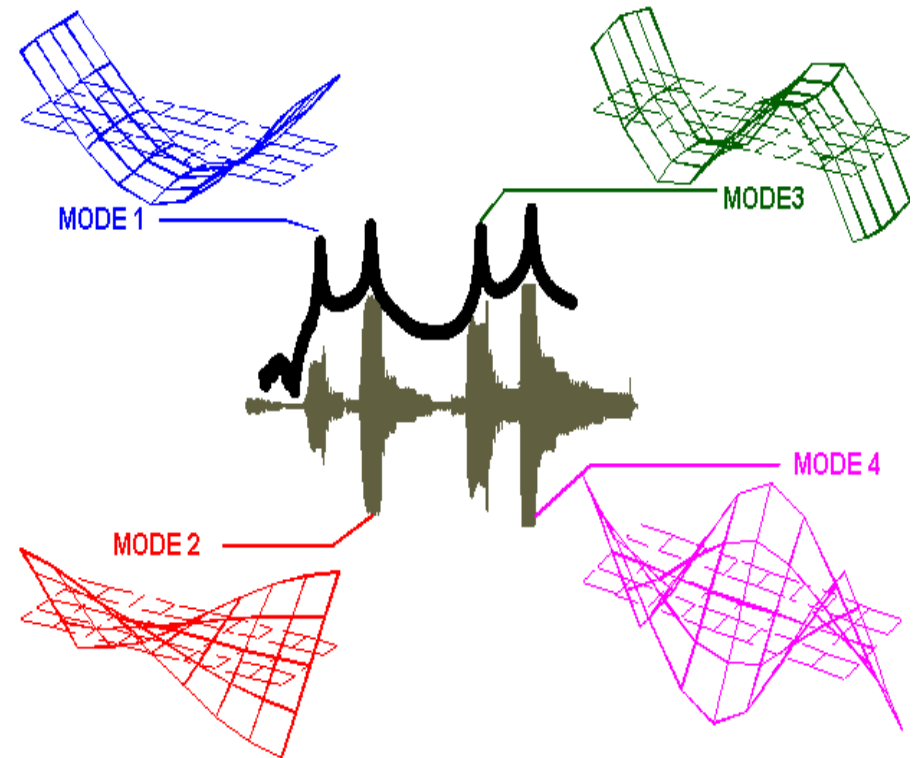
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GOSPODARSTWA  
REGIONALNEGO

The method was introduced in 1982 by Baruh and Meirovitch

**The modal filter extracts modal coordinates from measurements of the system response.**

**It can be applied for:**

- ✓ dealing with the control spillover problem,
- ✓ the vibration control of flexible structures,
- ✓ correlation analysis for experimental and analytical modal vectors,
- ✓ identification of operational forces from the system response,
- ✓ damage detection



**Filtering of environmental changes (temperature changes) influence on structural dynamic properties of a structure**

# Modal filters - reciprocal modal vectors

To construct the discrete modal filter from experimental data, a new type of modal parameters has to be introduced – reciprocal modal vectors  $\psi_r$ . They should be orthogonal with respect to the modal vectors, and thanks to that, they are applied to decomposition of the system responses to the modal coordinates  $\eta_r$ .

$$\eta_r \psi_r^T = \Psi_p^T \cdot \left( \frac{\phi_r^T}{j\omega - \lambda_r} + \psi_r^T \phi_r^* \frac{\phi_r^{*T}}{j\omega - \lambda_r^*} \right)$$

the assumption that the modal residue  $R_{rpp}$  is in the imaginary form:

$$R_{rpp} = j \cdot 1$$

the 1 DOF frequency response function  $H_{pp}(\omega)$

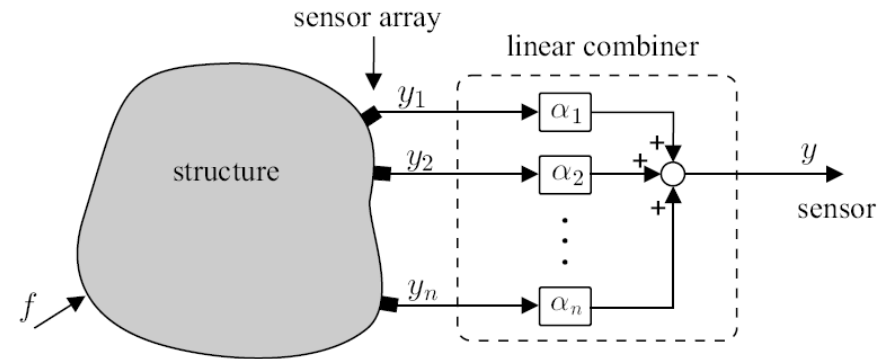
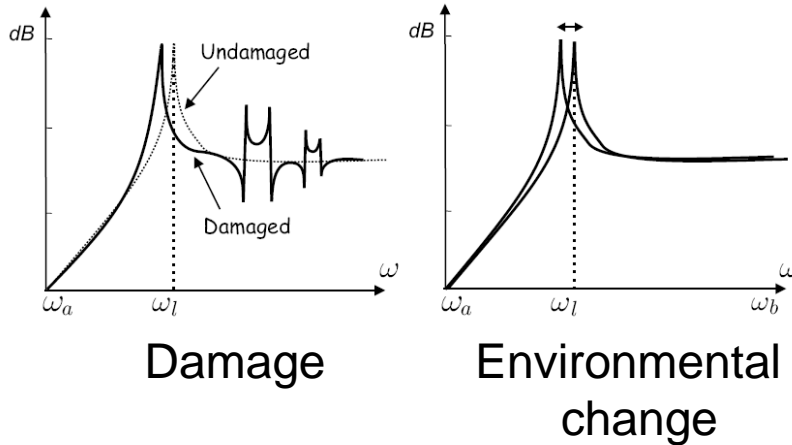
$$H_{pp}(\omega) = \frac{R_{rpp}}{j\omega + \lambda_r} + \frac{R_{rpp}^*}{j\omega + \lambda_r^*}$$

the experimental frequency response functions  $k \times N$  matrix:

$$H_{kN} = \begin{bmatrix} H_1(\omega_1) & H_2(\omega_1) & \dots & H_N(\omega_1) \\ H_1(\omega_2) & H_2(\omega_2) & \dots & H_N(\omega_2) \\ \vdots & \vdots & \ddots & \vdots \\ H_1(\omega_k) & H_2(\omega_k) & \dots & H_N(\omega_k) \end{bmatrix}$$

reciprocal modal vectors matrix  $\Psi_p$  determination:

$$\Psi_p = H_{kN}^+ \cdot H_{pp}$$



Local change of stiffness

$$M^{-1} \cdot (-K_u) \cdot \Phi = \Phi \cdot \Lambda$$

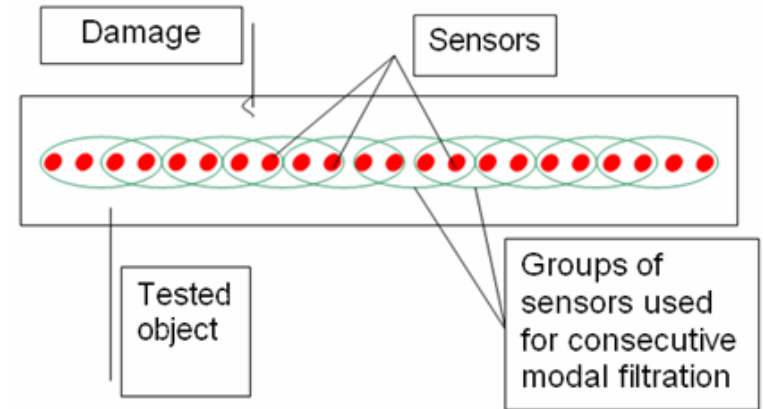
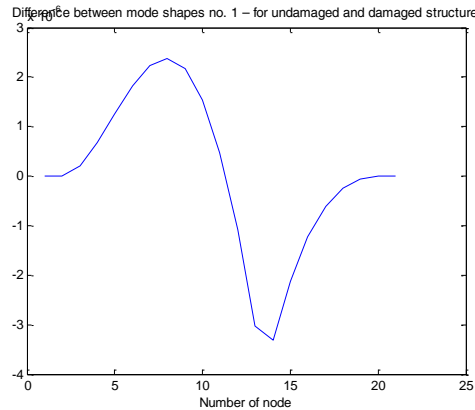
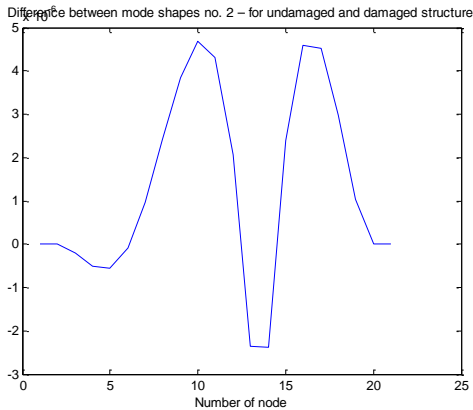
$$K_d = K_u \pm \Delta K \quad K_t = \alpha \cdot K_u$$

$$M^{-1} \cdot (-\alpha \cdot K_u) \cdot \Phi_t = \Phi_t \cdot \Lambda_t$$

$$\Lambda_t = \alpha \cdot \Lambda \quad \text{and} \quad \Phi_t = \Phi$$

**Conclusion: any method based on the modal vectors is resistant for environmental changes**

## Differences between mode shapes of damaged and undamaged beam with 10 % crack

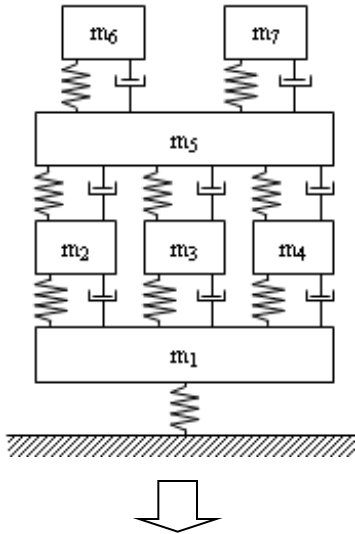


## Procedure

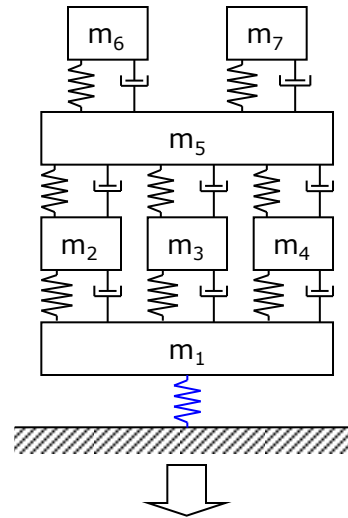
- Preliminary calculations performed for an object in the reference (undamaged) state – modal analysis, calculation of Reciprocal Modal Vectors for all sensors and for sensors from consecutive areas.
- Measurements of object characteristics in their current state and their filtration with the use of a “global” modal filter.
- In the case of damage detection in Point 2 of the procedure, filtration of groups of characteristics from consecutive object areas with the use of “local” modal filters.
- Calculation of the damage index DI for each of the sensor regions.

$$DI_4 = \frac{\int_{\omega_s}^{\omega_f} |x_i \langle \psi | - x_{ref} \langle \psi | |^2 d\omega}{\int_{\omega_s}^{\omega_f} |x_{ref} \langle \psi | |^2 d\omega}$$

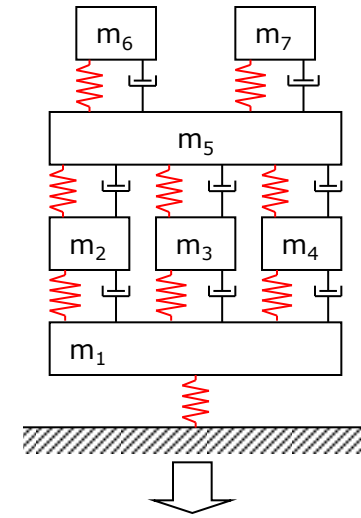
# Numerical verification - model



Damage  
 5% stiffness drop in  
 spring no. 1



Temperature change  
 5% stiffness drop in  
 all springs



Modal model  
 (natural frequencies,  
 modal damping coeff.,  
 modal vectors)

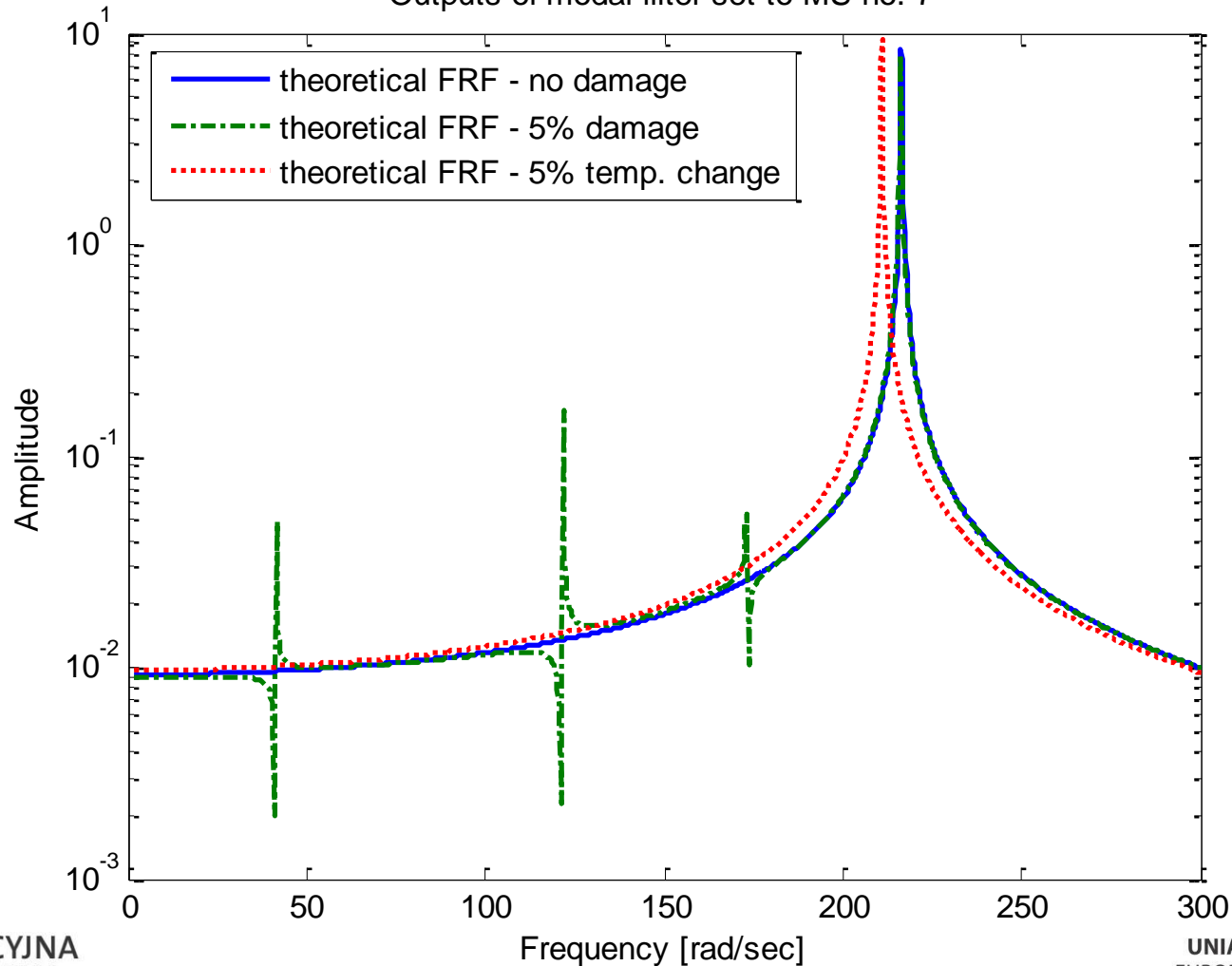
FRF synthesis

Modal filter  
 (reciprocal modal  
 vectors)

Modal filtering

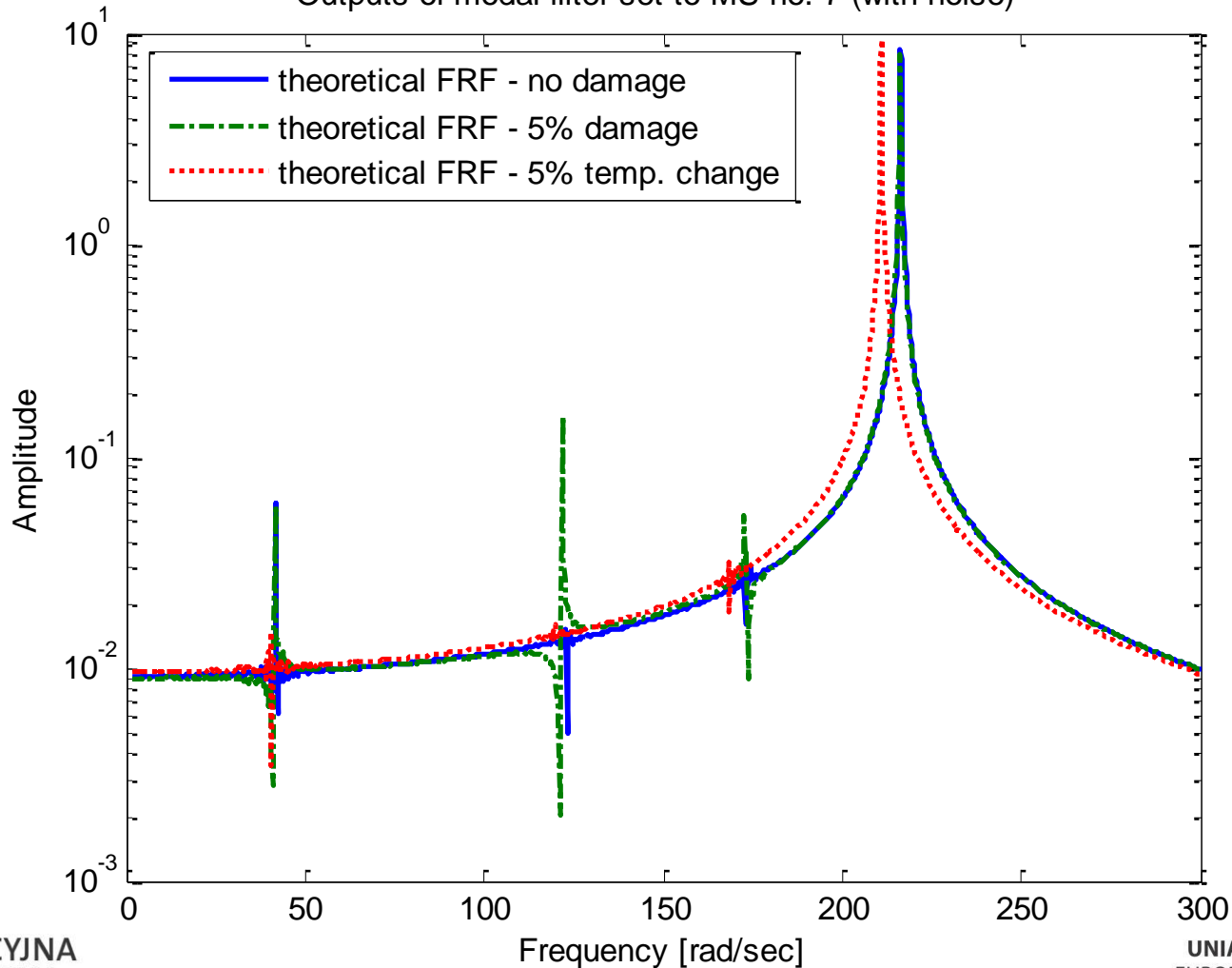
# Numerical verification - results

Outputs of modal filter set to MS no. 7



# Numerical verification - results

Outputs of modal filter set to MS no. 7 (with noise)

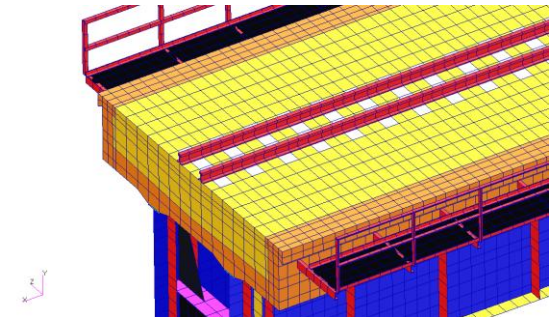
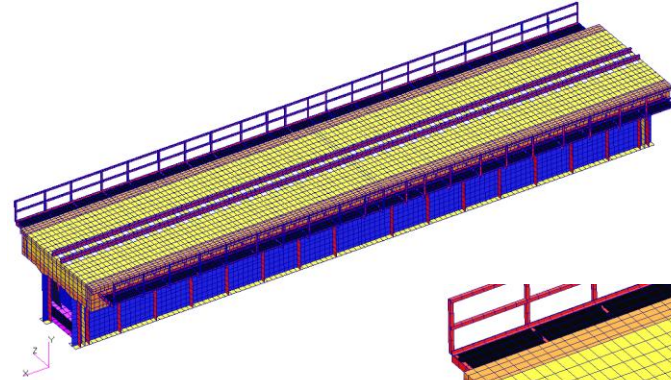
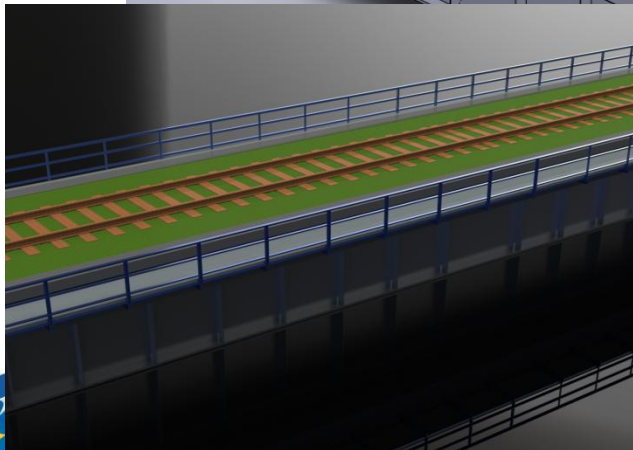
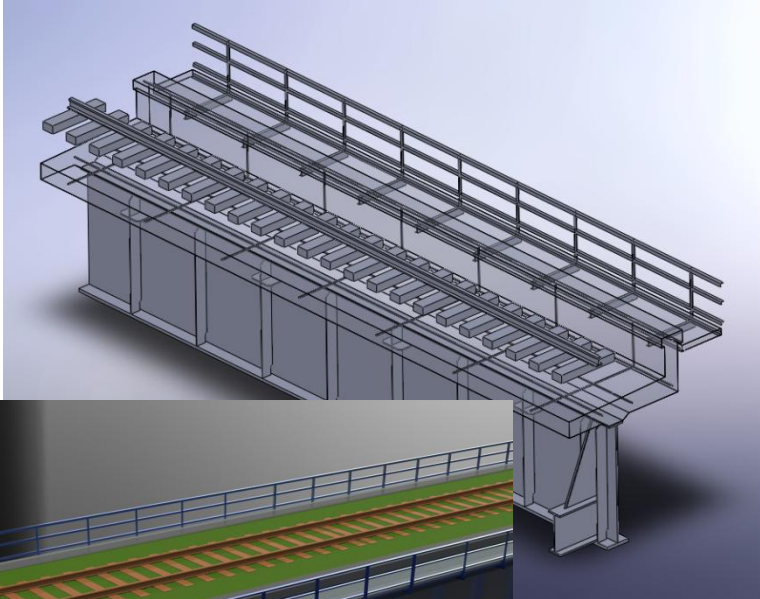




## Model of the rail viaduct

CAD model was based on documentation of real structure. Bridge is 27 m long and consists of steel (beams, barriers, rails, reinforcement of main plate), concrete (main plate, pavements), soil (sub-crust) and wood (sleepers) elements.

FEM model was built. It consists of approximately 28500 elements and 30500 nodes. Solid, shell and beam elements were used in the model



## Model of the rail viaduct

There were three cracks introduced in the model. First and second are vertical cracks in web, third is flange crack. Crack localization was based on linear-static stress analysis.

	Steel (20°C)	Wood	Concrete	Soil
E	$2.1 \cdot 10^5 \text{ MPa}$	$0.11 \cdot 10^5 \text{ MPa}$	$32.4 \cdot 10^{-3} \text{ MPa}$	10 MPa
ν	0.3	0.06	0.167	0.15
ρ	$7.85 \frac{\text{g}}{\text{cm}^3}$	$0.8 \frac{\text{g}}{\text{cm}^3}$	$2 \frac{\text{g}}{\text{cm}^3}$	$2 \frac{\text{g}}{\text{cm}^3}$

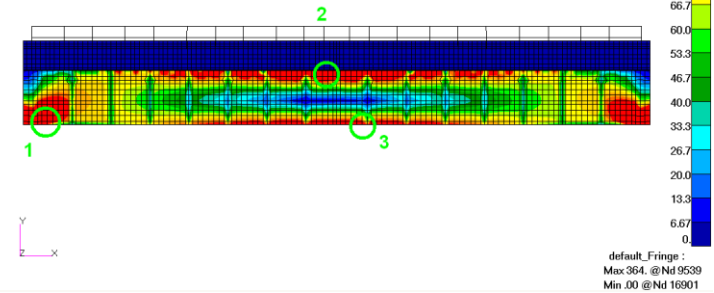
To take into account influence of moisture, different material densities were used

Moisture (W)	Density
15 %	$0.92 \frac{\text{g}}{\text{cm}^3}$
25 %	$1 \frac{\text{g}}{\text{cm}^3}$

Moisture (W)	Density
5 %	$2.1 \frac{\text{g}}{\text{cm}^3}$
8 %	$2.16 \frac{\text{g}}{\text{cm}^3}$

Patran 2008r128-Aug 09 20:41:03

Fringe: STATIC, A1:Static Subcase, Stress Tensor, , von Mises, Maximum,3 of 3 layers



The following normal modes analysis

scenarios were considered:

Without crack (dry)

Without crack (moist)

Without crack (wet)

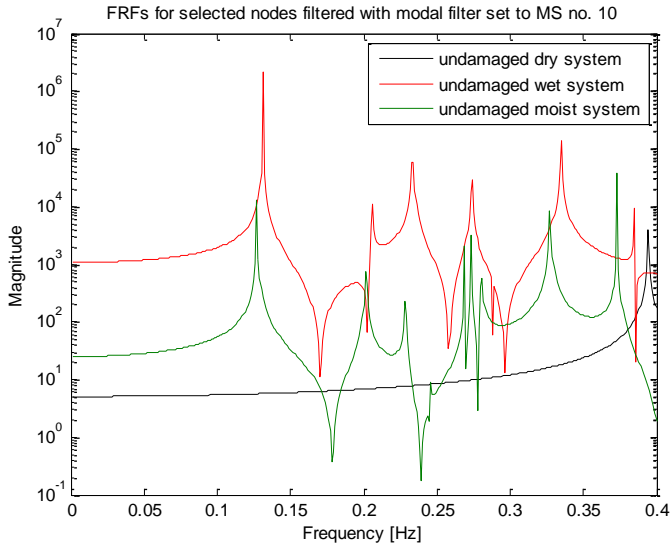
Crack 1

Crack 2

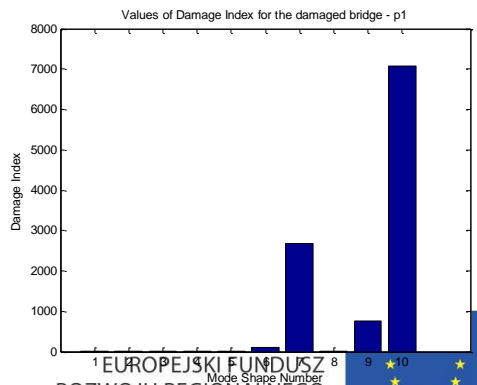
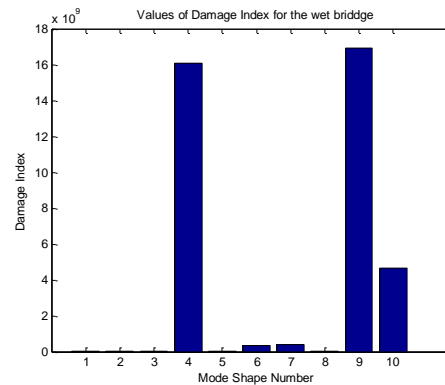
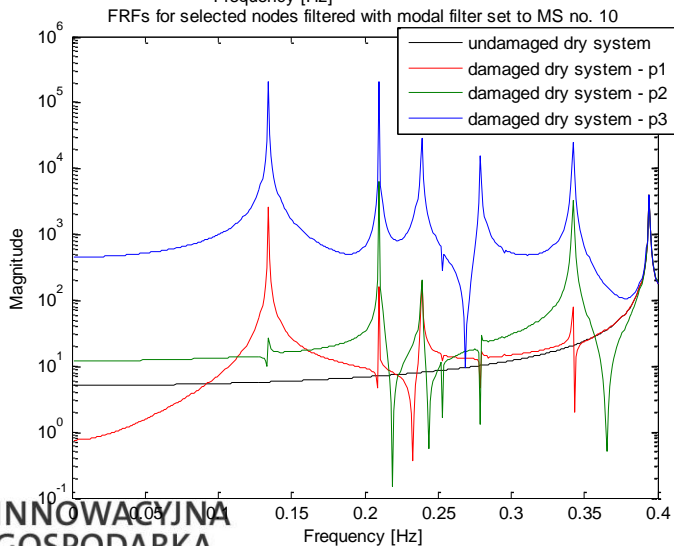
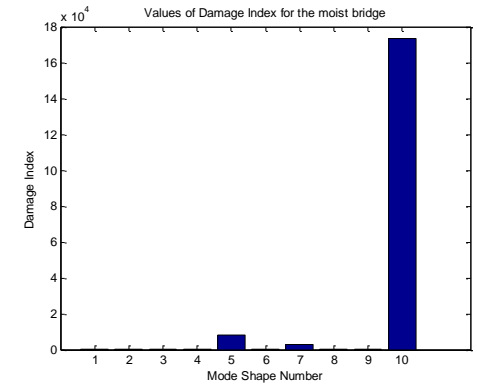
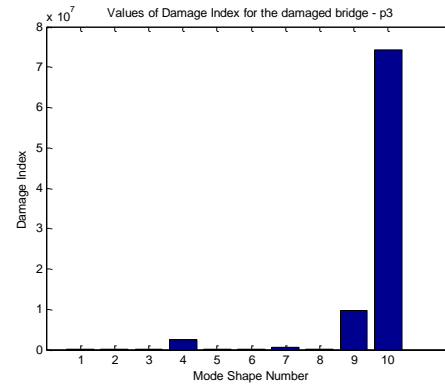
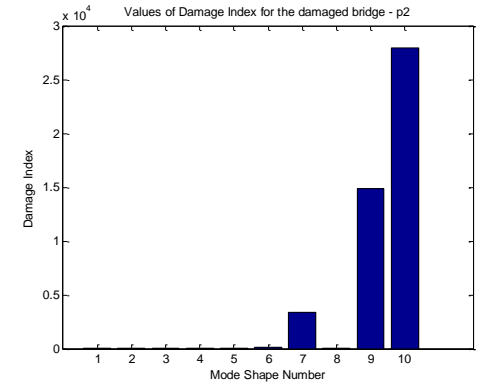
Crack 3

Crack 123

# Humidity - results



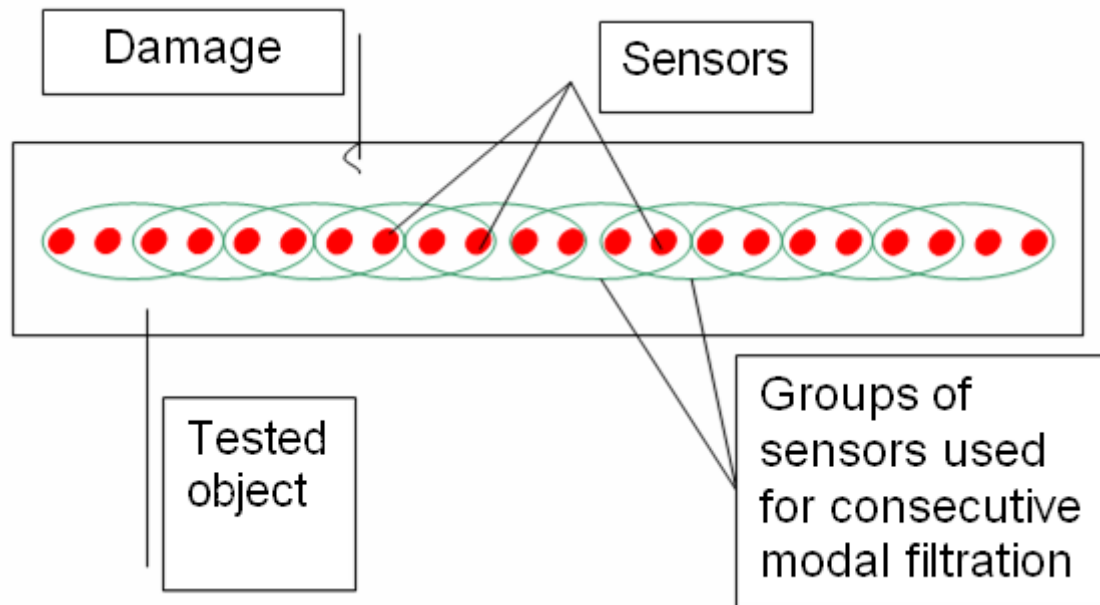
$$DI_4 = \frac{\int_{\omega_s}^{\omega_f} |x_i(\omega) - x_{ref}(\omega)|^2 d\omega}{\int_{\omega_s}^{\omega_f} |x_{ref}(\omega)|^2 d\omega}$$



## Damage localization

The idea for extension of the method by adding damage localization, bases on two facts:

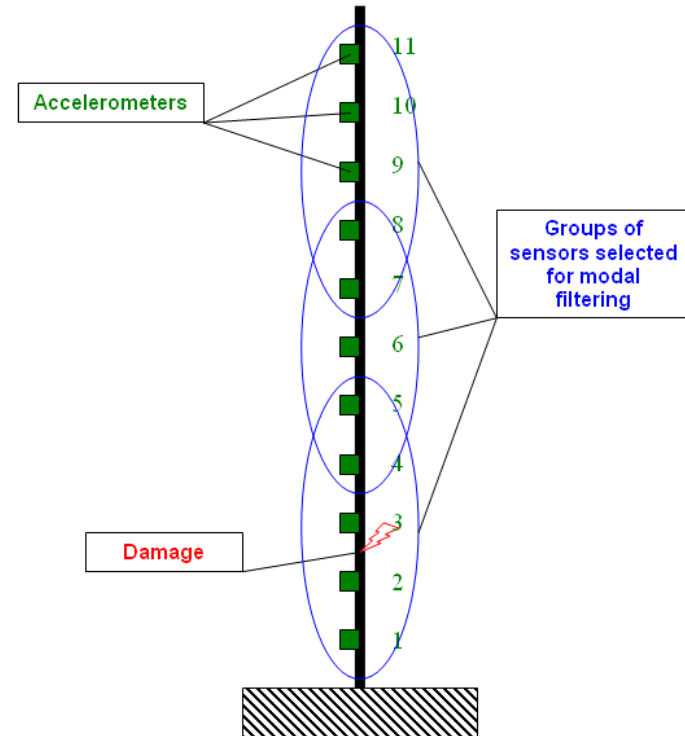
1. Effective modal filter can be formed for just a few sensors (in practice about 4 - 5),
2. That damage, in most of the cases, disturbs the mode shapes (modal vectors) only locally.



# Experimental verification – laboratory stand

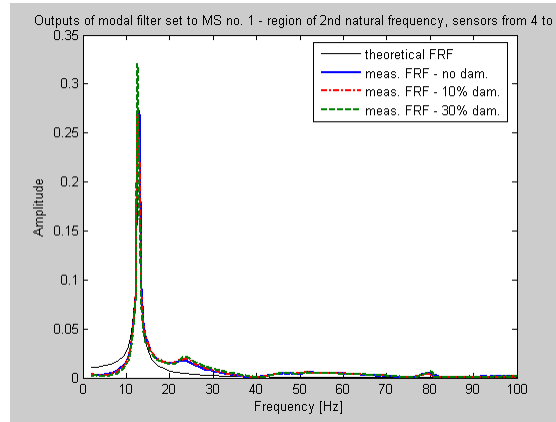
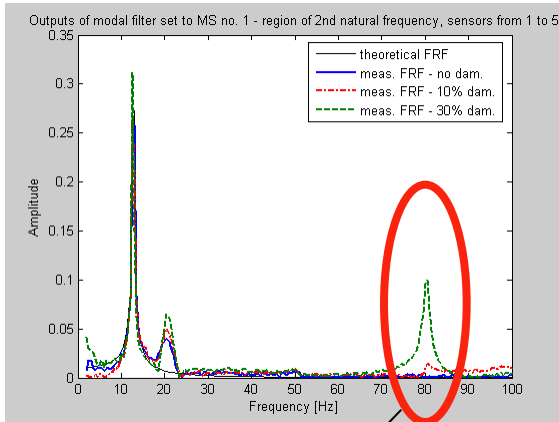


Three modal tests were performed, first on the undamaged beam. Next the cut between measuring points 2 and 3 was introduced. The magnitude of the cut amounted consecutively 10 % and 30 % of the beam cross-section area.

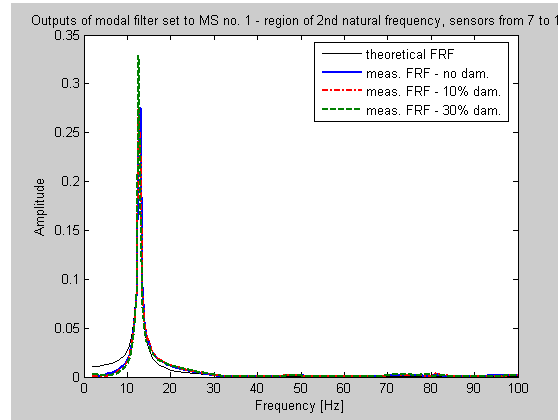




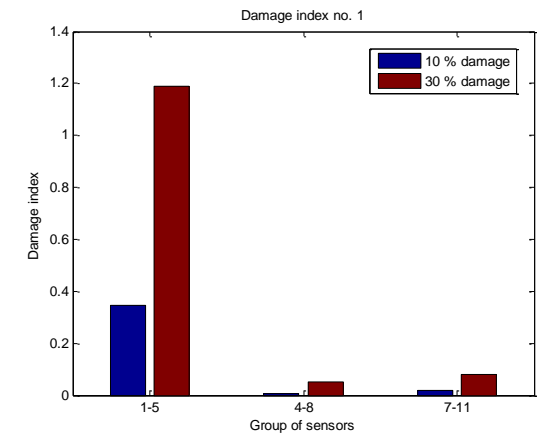
# Experimental verification- results



damage location

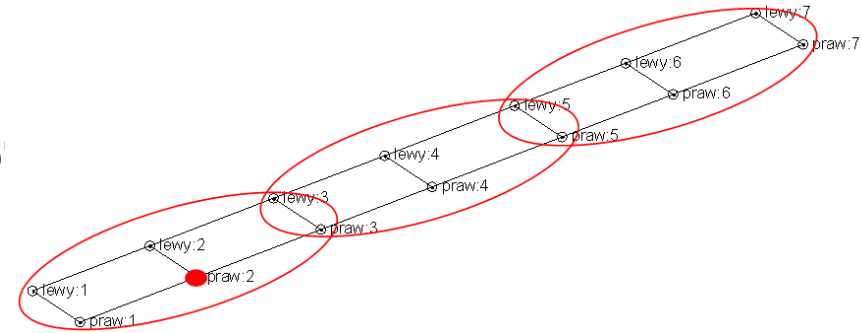


$$DI_4 = \frac{\int_{\omega_s}^{\omega_f} |x_i(\omega) - x_{ref}(\omega)|^2 d\omega}{\int_{\omega_s}^{\omega_f} |x_{ref}(\omega)|^2 d\omega}$$





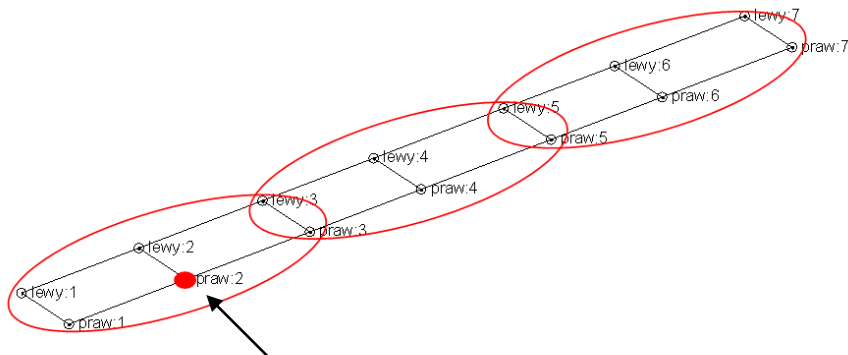
S  
O



Experimental modal analysis with impact excitation

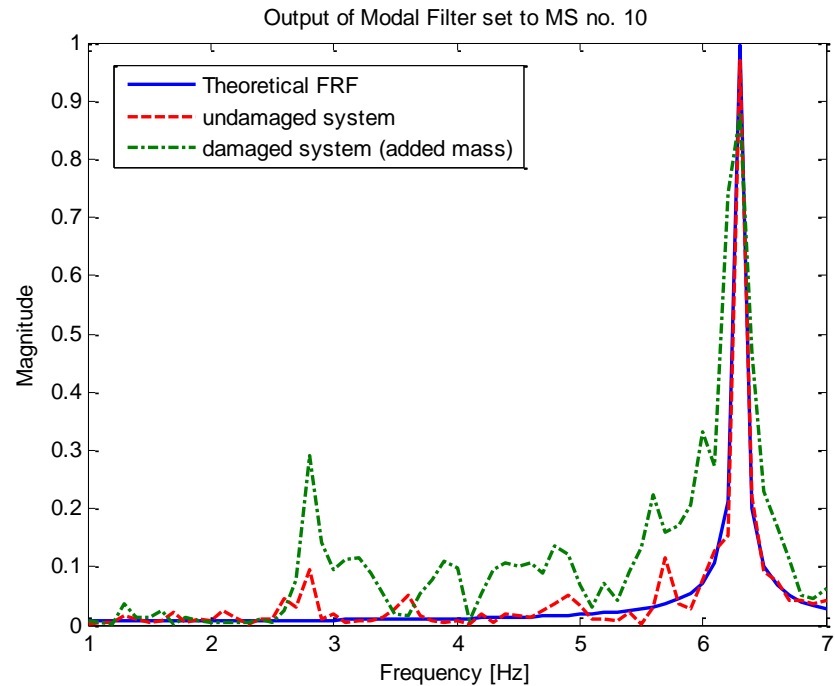
Nr CzDW	CzDW [Hz]	WTM [%]
1	1.35	0.36
2	1.59	1.29
3	2.06	0.13
4	2.82	0.50
5	3.01	1.77
6	3.66	0.03
7	4.99	1.22
8	5.15	1.26
9	5.62	1.02
10	6.29	0.33

# Case studies – modal filter for footbridge



Added mass - about 200 kg

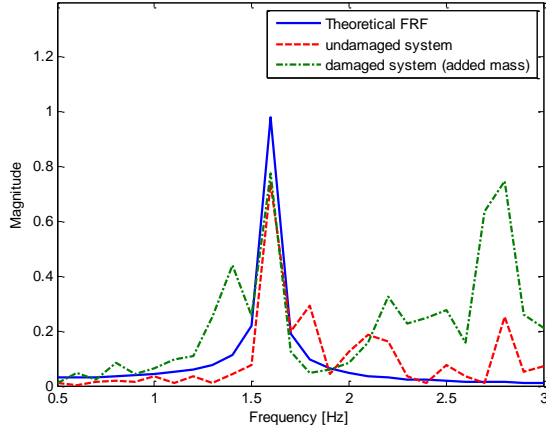
## Ambient excitation



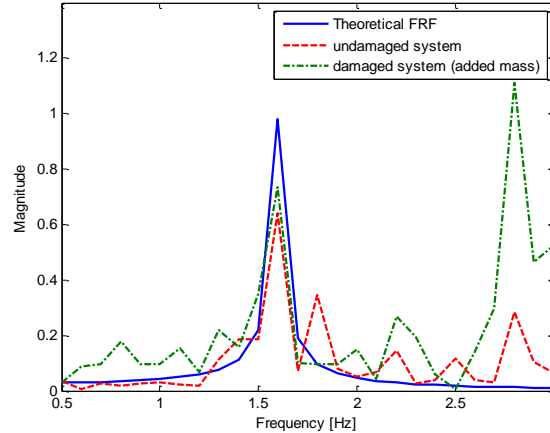


# Case studies – modal filter for footbridge

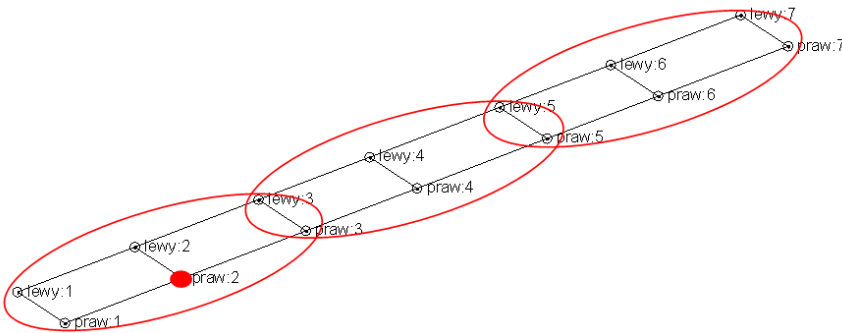
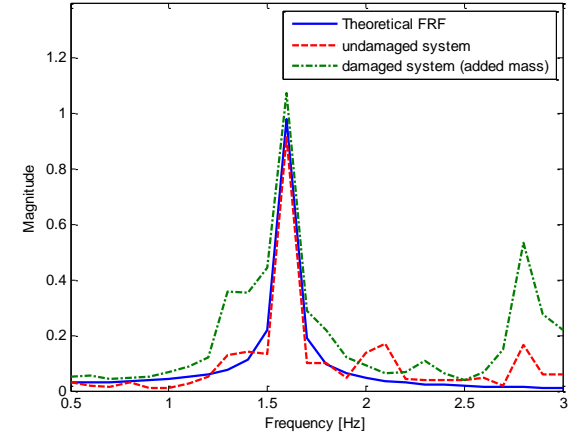
Modal Filter output for selected points (Group no. 1) filtered with filter set to MS no. 2



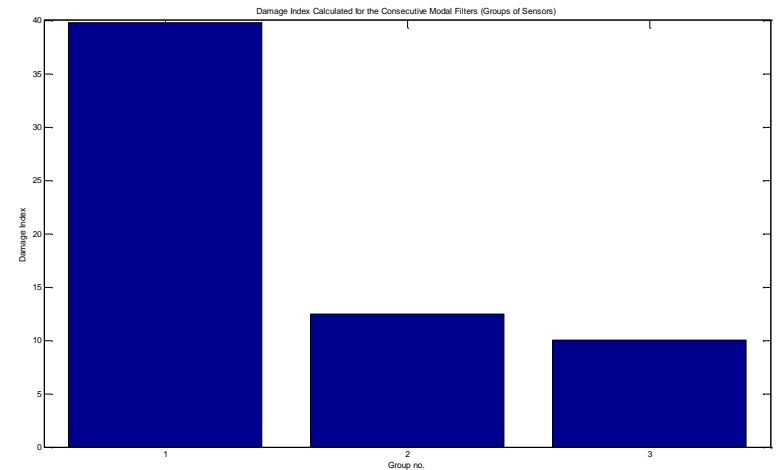
Modal Filter output for selected points (Group no. 2) filtered with filter set to MS no. 2



Modal Filter output for selected points (Group no. 3) filtered with filter set to MS no. 2



Added mass - about 200 kg



# Badania kładki dla pieszych nad ulicą Opolską dla Zarządu Infrastruktury Komunalnej i Transportu w Krakowie

Obiekt badań: **kładka dla pieszych nad ulicą Opolską.**

Rodzaj kładki: podwieszona, dla pieszych;

Ilość przęseł: 2 podwieszone + część podparta;

Rozpiętość: 39,00 m + 39,00 m + 42,00 m;

Pylon: stalowy, typu A; Wysokość pylonu: 34,52 m;

Układ nośny: układ podwieszony, promienisty, zbieżny;

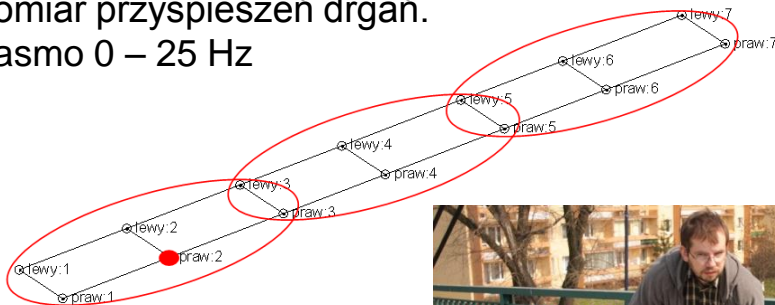
**Przeprowadzono dwa testy modalne,**

Wymuszenie impulsowe

Sieć 14 punktów pomiarowych

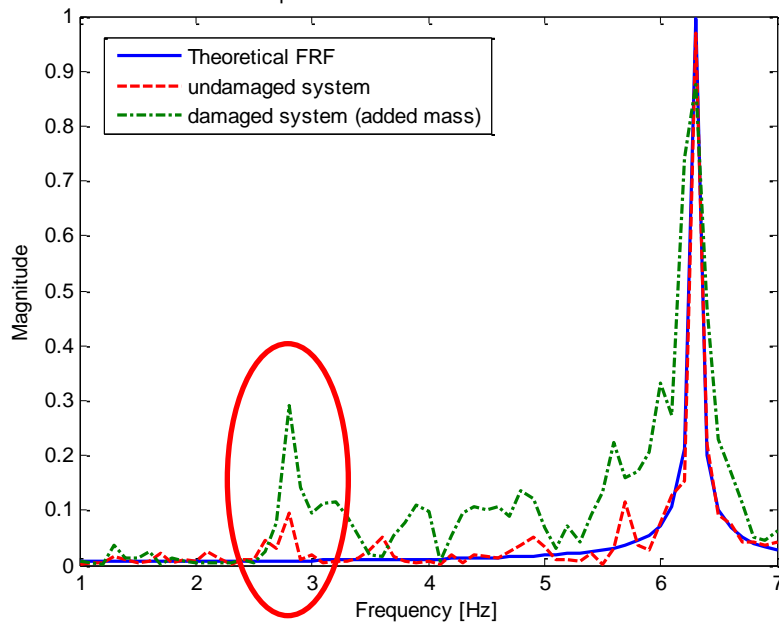
Pomiar przyspieszeń drgań.

Pasmo 0 – 25 Hz



# Badania kładki dla pieszych nad ulicą Opolską dla Zarządu Infrastruktury Komunalnej i Transportu w Krakowie

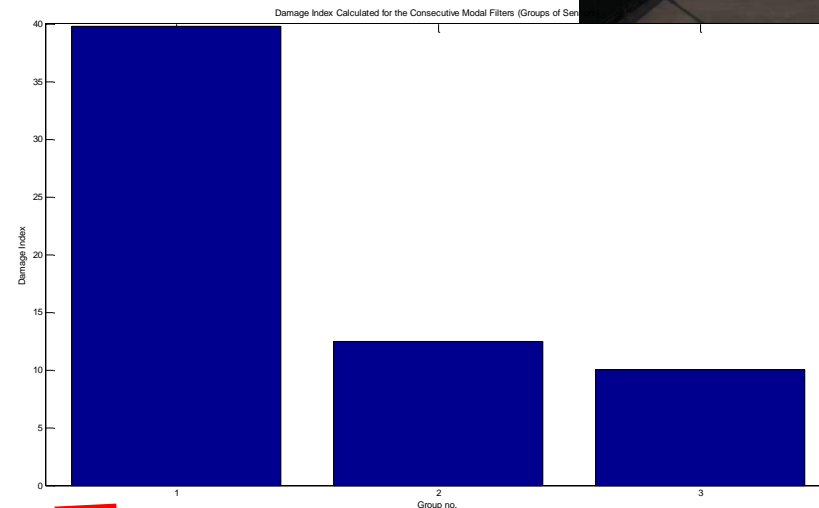
Output of Modal Filter set to MS no. 10



Uszkodzenie symulowane  
było poprzez dodanie  
masy około 200 kg  
zlokalizowanej w punkcie  
pomiarowym Praw:2.  
(około 5% masy obiektu w  
najbliższym otoczeniu).



Zasymulowane uszkodzenie zostało  
poprawnie wykryte i zlokalizowane



**Rejon uszkodzenia**

# Conclusions

**SHM systems improve safety of operation of critical structures**

**Long term monitoring is required**

**Local methods are more sensitive than global one**

**SHM systems are expensive (local system)**

**Detection and localization, assessment are relatively easy**

**Prediction is a very difficult task**

**Interdisciplinary approach is required**

**SHM systems commercial use in civil structures, aviation, space, railway, power plants.**